

# **“STUDI STANDARISASI REDESAIN STRUKTUR BAJA MENJADI STRUKTUR BETON TAHAN GEMPA”**

**(Studi Kasus : Gedung Kuliah Kampus Unej Cabang Bondowoso)**

Harsono

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## **RINGKASAN**

Redesain adalah kegiatan merancang kembali suatu bangunan dengan mengubah material utama struktur tanpa merubah fungsi dan lokasinya. Gedung Kuliah Kampus Unej Cabang Bondowoso merupakan konstruksi bangunan yang terdiri dari 5 lantai yang mana struktur utamanya adalah struktur baja. Pada perhitungan ini mengacu pada beberapa peraturan, diantaranya yaitu SNI 1726-2019, SNI 03-1729-2002, dan PPIUG 1987. Analisis gaya-gaya dalam yang timbul pada elemen struktur menggunakan program bantu SAP 2000 v22. Dari perhitungan yang telah dilakukan, maka diperoleh kesimpulan, yaitu Pertama kekakuan struktur dengan mengambil acuan periode getar alami struktur ( $T$ ), didapatkan nilai  $T$  baja = 2,122233 detik >  $T$  beton = 0,72034 detik. Kekuatan kolom menghasilkan interaksi aksial-momen,  $K1$  baja = 0,914 < 1,  $K2$  baja = 0,705 < 1 dan  $K1$  beton = 0,61 < 1,  $K2$  = 0,39 < 1. Kekuatan penampang balok didapatkan nilai maksimum luas total tulangan tekan dan tarik yaitu, balok B1 As SAP2000 = 1276 mm<sup>2</sup> < As hitung = 1459 dan balok B2 As SAP2000 = 186 mm<sup>2</sup> < As hitung = 392 mm<sup>2</sup>. Kedua hasil dimensi struktur redesign kolom K1 baja menjadi 500x300 dengan tulangan (8 D 19), kolom K2 baja menjadi 450x300 dengan tulangan (8 D 19), Balok B1 baja menjadi 350x300 dengan tulangan tekan dan Tarik (7 D 13) dan (4 D 13), balok B2 baja menjadi 200x150 dengan tulangan tekan dan Tarik (3 D 10) dan (2 D 10).

Kata Kunci: Gempa, Struktur baja, redesign.

**“STUDY OF STEEL STRUCTURE REDESIGN STANDARDIZATION  
BECOME A EARTHQUAKE-RESISTANT CONCRETE STRUCTURE”  
(Case Study : Bondowoso Branch Unej Campus Lecture Building)**

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**ABSTRACT**

*Redesign is the activity of redesigning a building by changing the main material of the structure without changing its function and location. Bondowoso Branch Unej Campus Lecture Building is a building construction consisting of 5 floors where the main structure is a steel structure. This calculation refers to several regulations, including SNI 1726-2019, SNI 03-1729-2002, and PPIUG 1987. Analysis of internal forces that arise in structural elements using the SAP 2000 v22 auxiliary program. From the calculations that have been carried out, it is concluded that the first is the stiffness of the structure by taking the reference period of the natural vibration of the structure (T), the steel T value = 2.122233 seconds > concrete T = 0.72034 seconds. Column strength resulted in axial-moment interaction, K1 steel = 0.914 < 1, K2 steel = 0.705 < 1 and K1 concrete = 0.61 < 1, K2 = 0.39 < 1. The beam cross-sectional strength obtained the maximum value of the total area of compression reinforcement and tensile ie, beam B1 As SAP2000 = 1276 mm<sup>2</sup> < As count = 1459 and beam B2 As SAP2000 = 186 mm<sup>2</sup> < As count = 392 mm<sup>2</sup>. The results of the structural dimensions of the redesigned K1 steel column became 500x300 with reinforcement (8 D 19), K2 steel column became 450x300 with reinforcement (8 D 19), B1 steel beam became 350x300 with compressive and tensile reinforcement (7 D 13) and (4 D 13), B2 steel beams to be 200x150 with compression and tension reinforcement (3 D 10) and (2 D 10).*

*Keywords: Earthquake, Steel structure, redesign.*